



Company Name	Pythons & Co	Project Title	A simple block of flats
Group/Team Name	Flying Circus	Subtitle	Abattoir
Designer	Mr. Wiggin	Job Number	1.1.3.3.2
Date	20 /06 /2018	Client	Mr. Tid

Design Conclusion

Cleat Angle	Fail
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Cleat Angle

Connection Properties

Connection

Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection

Connection Category

Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted

Loading (Factored Load)

Shear Force (kN)	100
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Components

Column Section	WPB 280x280x61.2
Material	Fe 410
Beam Section	NPB 220x110x29.4
Material	Fe 410
Hole	STD
Cleat Section	100 100 x 8
Thickness (mm)	8
Cleat Leg Size B (mm)	100
Cleat Leg Size A (mm)	100
Hole	STD

Bolts on Beam

Type	Friction Grip Bolt
Grade	10.9
Diameter (mm)	20
Bolt Numbers	3
Columns (Vertical Lines)	1
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	40

Edge Distance (mm)	40
Bolts on Column	
Type	Friction Grip Bolt
Grade	10.9
Diameter (mm)	20
Bolt Numbers	6
Columns (Vertical Lines)	1
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	40
Edge Distance (mm)	40
Assembly	
Column-Beam Clearance (mm)	15.0



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Design Preferences

Bolt	
Hole Type	Over-sized
Material Grade (MPa) (overwrite)	800.0
Slip factor	0.52

Detailing	
Type of Edges	Rolled, machine-flame cut, sawn and planed
Minimum Edge-End Distance	1.5 times the hole diameter
Gap between beam & support (mm)	15.0
Are members exposed to corrosive influences?	No

Design	
Design Method	Limit State Design



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Design Check: Secondary Beam Connectivity

Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsf} = ((0.52 \cdot 2 \cdot 0.85 \cdot 137.2) / (1.25)) = 121.285$ [cl. 10.4.3]	
Bolt bearing capacity (kN)		N/A	
Bearing capacity of beam web (kN)		N/A	
Bearing capacity of cleat (kN)		N/A	
Bearing capacity (kN)		N/A	
Bolt capacity (kN)		121.285	
Critical bolt shear (kN)	≤ 121.285	34.319	Pass
No. of bolts		3	
No. of column(s)	≤ 2	1	
No. of bolts per column		3	
Bolt pitch (mm)	$\geq 2.5 \cdot 20 = 50, \leq \text{Min}(32 \cdot 6.6, 300) = 212$ [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	$\geq 2.5 \cdot 20 = 50, \leq \text{Min}(32 \cdot 6.6, 300) = 212$ [cl. 10.2.2]	0	
End distance (mm)	$\geq 1.5 \cdot 24.0 = 36, \leq 12 \cdot 6.6 = 79.2$ [cl. 10.2.4]	40	Pass
Edge distance (mm)	$\geq 1.5 \cdot 24.0 = 36, \leq 12 \cdot 6.6 = 79.2$ [cl. 10.2.4]	40	Pass
Block shear capacity (kN)	≥ 100	$V_{db} = 108.114$ [cl. 6.4.1]	Pass
Cleat height (mm)	$\geq 0.6 \cdot 222.0 = 133.2, \leq 222.0 - 10.2 - 1.2 - 10.0 - 2.4 - 5 = 193.2$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	180.0	Pass
Cleat moment		$M_d = (1.2 \cdot 250 \cdot Z) / (1000 \cdot 1.1)$	Pass

capacity (kNm)	$(2 \cdot 121.285 \cdot 50^2) / (50 \cdot 1000) = 3.0$	$= 77.76$ [cl. 8.2.1.2]	pass
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Design Check: Primary Beam Connectivity			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsf} = ((0.52 \cdot 1 \cdot 0.85 \cdot 137.2) / (1.25)) = 60.642$ [cl. 10.4.3]	
Bolt bearing capacity (kN)		N/A	
Bolt bearing capacity (kN)		N/A	
Bolt bearing capacity (kN)		N/A	
Bolt bearing capacity (kN)		N/A	
Bolt capacity (kN)		60.642	
Critical bolt shear (kN)	≤ 60.642	35.77	Pass
No. of bolts		6	
No. of column(s) per angle	≤ 2	1	
No. of bolts per column per angle		3	
Bolt pitch (mm)	$\geq 2.5 \cdot 20 = 50, \leq \text{Min}(32 \cdot 7.0, 300) = 224$ [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	$\geq 2.5 \cdot 20 = 50, \leq \text{Min}(32 \cdot 7.0, 300) = 224$ [cl. 10.2.2]	0	
End distance (mm)	$\geq 1.5 \cdot 24.0 = 36, \leq 12 \cdot 7.0 = 84.0$ [cl. 10.2.4]	40	Pass
Edge distance (mm)	$\geq 1.5 \cdot 24.0 = 36, \leq 12 \cdot 7.0 = 84.0$ [cl. 10.2.4]	40	Pass
Block shear capacity (kN)	≥ 100	$V_{db} = 108.114$ [cl. 6.4.1]	Pass
Cleat height (mm)	$\geq 0.6 \cdot 222.0 = 133.2, \leq 222.0 - 10.2 - 1.2 - 10.0 - 2.4 - 5 = 193.2$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	180.0	Pass

Cleat moment capacity (kNm)	$(2*60.642*50^2)/(50*1000) = 3.165$	$M_d = (1.2*250*Z)/(1000*1.1)$ $= 77.76$ [cl. 8.2.1.2]	Pass
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Views



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Additional Comments	
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